

Modeling bolts in LS-DYNA[®] using explicit and implicit time integration

Nils Karajan¹, Alexander Gromer¹, Thomas Borrvall², Kishore Pydimarry³

¹DYNAmore Corporation, 565 Metro Place South, Suite 300, Dublin, Ohio, 43017, USA

²DYNAmore Nordic, Brigadgatan 5, 58758 Linköping, Sweden

³Honda R&D Americas, Inc., 21001 State Route 739, Raymond, Ohio 43067, USA

Abstract

When setting up models for analysis using the explicit solver in LS-DYNA, the method of how to model bolted connections is usually well known. However, when this model or even just certain substructures of the model are used for load cases to be solved using the implicit solver in LS-DYNA, problems might arise that you might have not been aware of before. Typical automotive load cases for LS-DYNA implicit involve door sag, seat pull, misuse and other problems that are running over a long time span.

The goal of this contribution is to review the different modeling techniques for friction grip bolts in view of their respective spatial discretization, their required contact definitions, their pre-tension application and their load carrying behavior. Moreover, typical problems that arise during explicit and implicit time integration are discussed and solutions to these potential problems are provided. While these potential problems are often overseen in explicit, they become very apparent in implicit simulations when the user runs into convergence problems. This may especially be the case during the pre-tensioning phase of the bolt, when the friction grip connection starts to slip or ultimately fail.

Finally, a summary is given pointing out the current limitations as well as best practice approaches for four different types of modeling approaches for friction grip bolts, which can be downloaded from www.dynaexamples.com/connections/bolts.

Motivation

During the past three decades, LS-DYNA has been established as the standard solver for explicit analysis at many automotive, aerospace and defense companies worldwide. The predominant application always was and still is crash worthiness and other high-dynamic impact investigations. Hence, the typical LS-DYNA user has finite-element models that have been designed and built to run using the explicit solver of LS-DYNA.

Over time, the implicit capabilities of LS-DYNA have matured to a powerful simulation environment, which is well suited to solve problems with even several million elements as well as quasi-static or low-dynamic load cases with longer time spans. The benefit of switching to implicit are application specific and range from shorter simulation times in the case of very small element sizes to smoother response curves.

Following this, more and more users want to take their “explicit” models and simply use them with the implicit solver of LS-DYNA. This so-called one model strategy is possible to achieve but not all features of the explicit solver are available for implicit usage. To have a model working in explicit and implicit, certain modeling guidelines have to be followed. Sometimes, a material model or a specific element type is not yet available but ongoing code enhancements should narrow that gap even further in the future. However, the prevalent material models used in explicit are well supported by the implicit solver and even tricky contact situations can be mastered by applying the mortar contact option.

The goal of this contribution is to present four modeling strategies for friction grip connections with bolts and to point out possible pitfalls during the modeling process of such connections.

Friction Grip Bolts

Definitions and load bearing mechanism

This contribution is focused on bolted connections as shown in Figure 1a) where a threaded fastener with a head, a shank and an external male thread is pre-tensioned by a nut to join two or more sheets or blocks of material. Washers may be included to distribute pre-tensioning loads more evenly or to prevent the bolt to become loose but are neglected in this work for the sake of simplicity.

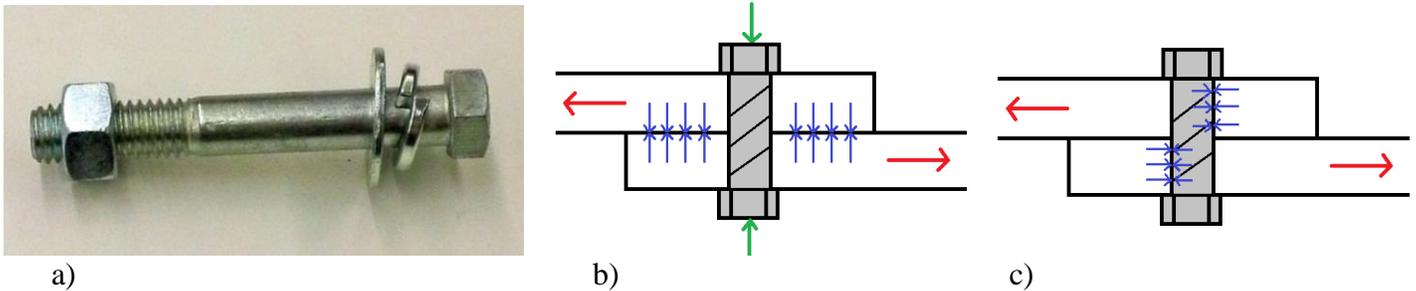


Fig. 1: a) Illustration of a bolt with its head, shank and thread as well as a washer disc to distribute stresses and a nut to pre-tension the connection b) load bearing behavior during service loads by friction and c) load bearing behavior for loading states beyond service load leading to bolt hole bearing (all pictures taken from Wikipedia).

The bolted connections under investigation are based on sufficient friction grip such that all service loads are carried by static friction between the connected sheets. Thus, applied service loads will not cause any relative motion or slip in the connection, cf. Figure 1b). To achieve a good grip, the bolts are typically pre-tensioned in the range of 75% or 90% of the proof load of the high strength bolt material for reusable or permanent connections, respectively. If the so-called friction grip bolt connection is loaded beyond working conditions, i. e. during a crash loading, the static friction may be overcome as is shown in Figure 1c) leading to a slip between the sheets such that the hole bearing behavior will govern the shear load of the bolt and thus, the ultimate load of the connection.

The failure of bolted connections at ultimate load can be dominated either by plate tear-out failure or by bolt shear fracture. As the implicit investigation of typical load cases like misuse or door sag do not involve failure of the bolted connections, the detailed failure modes are not investigated in this contribution. Instead, the interested reader is referred to the works of [1-5].

Modeling techniques for pre-tensioned bolts

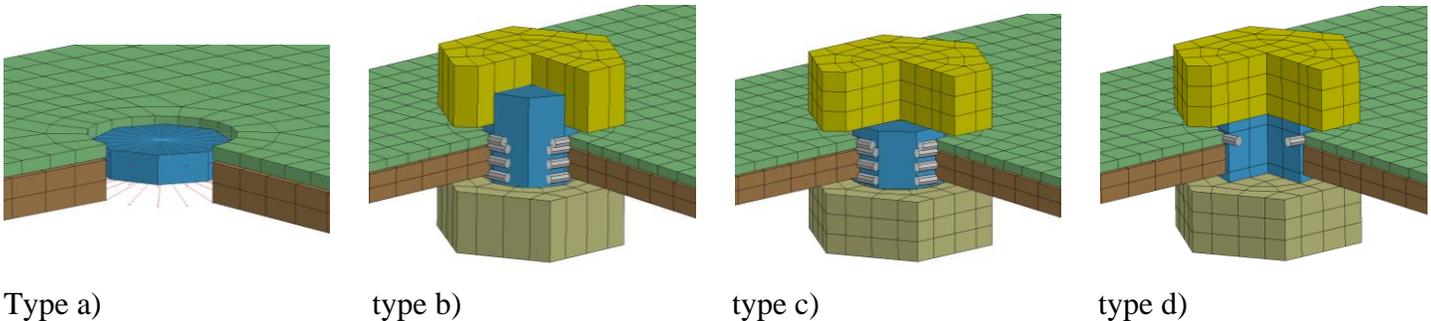
Meanwhile there are many different possibilities available how bolted connections can be modeled in LS-DYNA. The technique of choice depends on the loading stages that need to be captured, i.e. is it sufficient to capture loads in the friction grip regime or will slip or ultimate load be of interest, too. A good overview of modeling techniques can be found in *Sonnenschein* [1]. For the sake of completeness, we will briefly review the modeling possibilities for an explicit simulation and then highlight the necessary changes for successful implicit simulations. Benefits and merits of the respective modeling technique as well as possible pitfalls will also be summarized.

In general, there are two main modeling groups for pre-tensioned bolts. These can be distinguished by the discretization of the shank, which can be carried out using either beam or solid elements. If it is done using a beam element, it needs to be a “spot weld beam” (ELTYPE=9 in *SECTION_BEAM), which has to be used to-

gether with `*MAT_SPOTWELD` to be able to use `*INITIAL_AXIAL_FORCE_BEAM` to apply a pre-tension in the bolt. In cases where the shank is modeled with solid elements, a pre-stress is introduced via `*INITIAL_STRESS_SECTION`.

These two groups are further subdivided depending on the discretization of the head and the nut, which can be done using beam spiders, shell elements or solid elements. Keep in mind that shell elements have no rotational degrees of freedom around the shell normal. Thus, the bolt is not able to carry torsion loads, if the shank is discretized with a beam element and the nut as well as the bolt head is modeled with shell elements. Similar holds for a shank made of beam-elements that is connected to a head and a nut discretized by solid elements.

Different combinations of these possibilities allow for several modeling techniques. The techniques shown in Fig. 2 are frequently used but variants of those four types can of course also be used.



Type a)

type b)

type c)

type d)

Fig. 2: Illustration of the presented bolt modeling techniques type a) to d).

Bolt Modeling Techniques in Detail

Type a) Beam element for shank and beam-spider mesh connection to connected parts

General remarks

The shank is represented by one beam element while the head and nut (including washers) are represented by circular beam spider meshes that connect to the perimeter nodes of the bolt holes in the connected sheet metals or blocks. Herein, the beam spider mesh can represent either deformable beam elements or simply a nodal rigid body. In the case of two connected parts, there is typically no need to define a contact interface between the bolt shank and the bolt holes as well as the bolt head or nut and the outside clamped parts. To capture the contact between the connected materials, a `*CONTACT_AUTOMATIC_SINGLE_SURFACE` definition is sufficient. If a third sheet or block is present in the connection, please refer to the type b) connection on how to model its contact with the shank.

Explicit vs. implicit time integration

During implicit runs, it is generally recommended to use the `_MORTAR` option in the contact definition, to achieve a better convergence behavior. Thus, the switch to implicit simulations is straightforward and it is usually sufficient to add the `_MORTAR` option to the global contact definition of choice. This works fine in the case of two connected parts. For more connected parts where a slipping motion of the middle sheet might occur, please refer to connection type b).

Merits and drawbacks

This is the simplest and fastest way of modeling friction grip bolts to connect two parts consisting of shell or solid elements. A drawback of this modeling technique lies in its simplicity. Herein, a slipping motion can only be captured between the connected parts but never between the nut or the head and the connected parts, as the beam-spider mesh is sharing nodes with shell or solid elements of the connected parts. Following this, any slip

motion between the sheets directly activates the hole bearing behavior, which instantly loads the bolt in shear. Thus, the connection might behave too stiff for loads above the service load. In addition, if connection failure is of interest, the shear load in the bolt might be overestimated and the plate tear-out failure might not be captured correctly, either.

Moreover, the material model of the shank is limited to *MAT_SPOTWELD, which includes a bilinear plasticity law as well as the possibility to prescribe a piecewise linear hardening curve. However, accurate modeling of bolt failure may suffer from the one element per shank limitation. Thus, if shear failure in the bolt is of interest, the user is advised to apply modeling technique where the shank is represented by solid elements.

Type b) Beam element for shank and shell elements for nut and head

General remarks

As for the bolted connection type a), the shank is discretized by one beam element but the head and nut as well as washers are discretized by either deformable or rigid shell elements. In contrast to the bolted connection of type a), an additional contact interface needs to be created. This addresses the contact between the bolt head, the nut, the washers (if included) and the outside clamped parts, which is usually achieved by adding the part ID of head, nut and washers to the global *CONTACT_AUTOMATIC_SINGLE_SURFACE definition. In loading situations that never exceed the service loads, this will sufficiently model the friction grip bolt.

If the connection is loaded beyond service loads such that slipping or failure occurs, the contact between the shank and the bolt holes needs to be additionally defined to be able to capture the hole bearing behavior. This is because the typical *CONTACT_AUTOMATIC_SINGLE_SURFACE definition is not able to adequately capture the hole bearing behavior, as it is not capable of detecting beam-to-shell-edge or beam-to-beam interaction. Moreover, the interaction of a beam with a shell segment or a solid surface is only captured, if the nodes of the beam get in contact with the shell segment. However, if the nodes lay outside the segment, the contact of the beam element itself with the shell segment is not taken into account, as this type of contact only checks nodes against segments for penetration. This deficiency is overcome by additionally defining the numerically more expensive *CONTACT_AUTOMATIC_GENERAL_MPP or by switching on the _MORTAR option for the *CONTACT_AUTOMATIC_SINGLE_SURFACE definition, which is even more expensive.

For numerical efficiency, the experienced LS-DYNA user typically wants to limit the usage of these more expensive contact treatments to areas where it is really needed instead of referencing the whole part ID of the connected sheet metals. Following this, an extra part ID is created containing a set of contact null beams (ELTYPE=1 in *SECTION_BEAM), which are only attached to the nodes along the perimeter of the shell edges or solid faces of the bolt holes, cf. Fig. 2b) and 2c). Note that the contact null beams are typically modeled with a diameter equal to the respective shell thicknesses as well as *MAT_NULL with a very low density and reasonable values for Young's modulus and Poisson's ratio. Herein, the latter are only used for the computation of the penalty stiffness during the contact treatment and do not contribute to the "physical stiffness" of the sheet metal.

In explicit simulations, these null beams are brought into contact with the part ID of the shank using a separate *CONTACT_AUTOMATIC_GENERAL_MPP. The _MPP option allows setting the flag CPARAM8, which excludes beam-to-beam contact from the same part ID to avoid self-contact of the null beams. Moreover, as the spot weld (ELTYPE=9) beams are special beams, their contact treatment is only permitted if CPARAM8=2. Otherwise, spot weld beams are disregarded entirely by AUTOMATIC_GENERAL contacts. Note that the AUTOMATIC_GENERAL contact can also be applied without the _MPP option but then the spot weld beams need to be covered with contact null beams (ELTYPE=1), too, which need to be included in the contact part set.

Please note that by applying the _MORTAR option in the contact definition, the contact logic is switched from a node-to-surface penetration treatment to a segment-based penetration treatment, which additionally detects beam-to-beam as well as beam-to-shell-edge penetrations. Moreover, the typical tubular segment exten-

sions at the shell edges are switched off, i.e. the shell edges are then flat instead of round. The same can be achieved for the non-mortar contact definitions if `SHLEDG=1` is set in `*CONTROL_CONTACT`.

Explicit vs. implicit time integration

To switch an “explicit deck” to implicit time integration, simply add the `_MORTAR` option to the global `*CONTACT_AUTOMATIC_SINGLE_SURFACE`. To remain the same contact surfaces, the additional part ID of the contact null beams of the bolt hole can be included to the part set that is used in the single surface contact definition but the mortar contact treatment will also work without the null beams, which will then take place without the segment extensions at the shell edges. However, the spot weld (`ELTYPE=9`) beams that are used to discretize the shank were not included in the mortar contact of older LS-DYNA versions either. Thus, if there are problems, the bolt needs to be covered with contact null beams (`ELTYPE=1`), too. This deficiency has been recognized and LS-DYNA R9.2 is naturally able to treat spot weld beams in contact, while in R10 and R10.1, which were released earlier, this is still not working correctly.

Please note that in implicit simulations, the drilling rotation constraint for shell elements is switched on by default, which constrains the otherwise open torsional degree of freedom of the beam element in the shank when sharing nodes with flat shell element topologies like the head and the nut. This is especially important in quasi-static simulations to prevent a singular stiffness matrix, which is the result of the beam’s unconstrained torsion degree of freedom. During explicit simulations, one can get away without switching on the drilling rotation constraint, but users are advised to switch it on during explicit simulations, too, by defining an appropriate part set `DRCPSID` in `*CONTROL_SHELL`. For more information on drilling rotation constraints, please consult *Erhart & Borrvall* [7]. Please further note that the drilling rotation constraint will only help to overcome the singular stiffness matrix but is not suitable to transfer 100% of the physical torsion moment that might occur. If this is to be captured, a beam-spider mesh needs to be attached to the end nodes of the beam element of the shank as well as to the nodes of the shell elements of the head and nut, respectively.

Merits and drawbacks

If the application will stay within the service load regime and it is sufficient to capture the onset of a slipping motion, this modeling technique is very straightforward. Another benefit is that the critical time step in explicit simulations is typically not dominated by the connection, as the element size is not as small as in the bolt connection of type d). However, if the hole bearing behavior needs to be investigated, the modeling technique might be a little tedious to define. Moreover, if the bolt’s shear failure is of interest, this technique exhibits the same problems as the type a) connection. Thus, it might not be sufficient to accurately model shear failure of the bolt and the user is advised to apply modeling technique type d) to achieve results that are more accurate.

Type c) Beam element for shank and solid elements for nut and head

General remarks

This modeling technique is similar to type b) and is rather seldom used. If the head and nut are modeled with deformable solid elements, the same problems as in bolt connection type b) occur, i.e., the solid elements in the head and nut are not able to carry the torsional degrees of freedom of the beam element of the shank. Following this, the unconstrained degrees of freedom typically lead to a singular stiffness matrix during implicit time integration, which might prevent convergence.

In contrast to type b), it is not possible to switch on a drilling rotation constraint as is done for the shell elements in type b). Instead, one needs to include a beam spider mesh that attaches to some of the nodes of the solid elements of the head and the nut. In theory, one could also try to use special solid elements (`ELTYPE=3`) for the head and the nut that exhibit rotational degrees of freedom. However, for high pre tension forces, the attachment to only one node of the head and nut might lead to heavy deformation of the attached elements, as the force is very concentrated in that node. Thus, the force distribution via a beam-spider mesh is the better option.

If the head and nut are modelled with rigid solid elements, a special treatment in LS-DYNA takes care of connecting the torsional degree of freedom. This works straightforward even in LS-DYNA implicit.

Explicit vs. implicit time integration

The time step in explicit simulations will probably be limited by the relatively small deformable solid elements in the head and the nut, which is typically addressed using selective mass scaling on these part ID. Switching from explicit to implicit simulations leads to the same measures to be taken as described in connection type b).

Merits and drawbacks

From a computational view, going the extra mile and modeling the shank with solid elements, too, is well worth it, as simulation times will increase very little. Moreover, with this approach the tedious modeling work remains, which is why users tend to directly apply the bolt connection of type d) where the failure of the bolt can be captured more accurately.

Type d) Solid elements for shank, nut and head

General remarks

In this modeling technique, the shank is typically represented by meshes consisting of only hexahedron elements or a mixture of hexahedron and pentahedrons elements. Please note that tetrahedron elements are to be avoided, as a stress initialization of these elements is difficult to achieve on a satisfactory level. The connection to the head and nut is simply achieved by sharing nodes. If for any reason this is not desired, one can also work with a tied contact to stick together shank, head and nut on non-matching meshes.

From a modeling point of view, this technique is straightforward in cases where the connecting parts are also modeled with solid elements. Herein, all connection characteristics from service to failure loads are captured by simply adding the respective part ID of the shank, head and nut to the global *CONTACT_AUTOMATIC_SINGLE_SURFACE definition.

However, if the connecting parts are modeled with shell elements and loading situations beyond the service load are of interest, special attention is needed to capture the hole bearing characteristic and ultimately the failure load of the connection. Again, the different special treatments are necessary, as the *CONTACT_AUTOMATIC_SINGLE_SURFACE is not able to adequately capture shell-edge-to-solid-face interaction. Herein, the contact of the shell edge of the bolt hole with the solids of the shank is only captured when the nodes of the shell edge get in contact with the faces of the solid elements but not in between the nodes of the shell edge. Following this, there are three common ways to define the contact between the shaft and the bolt hole perimeter.

In the first approach, contact null beams are attached to the nodes along the perimeter of the shell edges of the bolt holes and brought into contact with the solid elements of the shank using either *CONTACT_AUTOMATIC_GENERAL(_MPP) or by activating the _MORTAR option in the contact definition, which is usually applied in explicit and implicit simulations, respectively. This approach is chosen in the presented example, as the meshed bolt hole size is compatible to connection type b) and c), i.e. the actual diameter is enlarged by the contact null beam diameter as well as it keeps compatibility between explicit and implicit simulations.

The second approach is an attempt to eliminate the need for contact null beams by switching to a contact definition with segment-based penetration checking for explicit and implicit simulations, i.e. *CONTACT_AUTOMATIC_SINGLE_SURFACE with SOFT=2 and the _MORTAR option, respectively. In the explicit case, one can usually omit the definition of contact null beams at the perimeter, as the segment extension at the shell edge mimics exactly the same behavior. Thus, SOFT=2 still preserves the compatibility of the meshed bolt hole size with the previously described bolt types. However, in the implicit case, the _MORTAR option does not have a segment extension such that comparability is lost without the introduction of contact null beams.

The third approach tackles the shortcomings of the second approach. Herein, the additional definition of `SHLEDG=1` in `*CONTROL_CONTACT` is defined to switch of the segment extension of the shell edges during explicit simulations using `*CONTACT_AUTOMATIC_SINGLE_SURFACE` with `SOFT=2`. Following this, compatibility of the contact situation implicit simulations is naturally given, as the `_MORTAR` option does not have a segment extension of the shell edge either. Thus, this allows for straightforward mesh generation of the bolt holes without the need to enlarge them by half of the diameter of the contact null beams. However, the compatibility to the other bolt modeling techniques is lost, which might not be a drawback when the other methods are not of interest.

Explicit vs. implicit time integration

The time step of the explicit simulation will probably be governed by the fine mesh of the bolt, which can be increased using selective mass scaling. A switch of this model to an implicit time integration is straightforward. Proceeding from the solid-element-only model case, all it needs is the additional `_MORTAR` option in the contact definition to boost the convergence behavior. In cases where the connecting parts are modeled using shell elements, it depends if contact null beams are present or not. In the presented example the part ID referenced in the `*CONTACT_AUTOMATIC_GENERAL(_MPP)` definition was included in the global `*CONTACT_AUTOMATIC_SINGLE_SURFACE_MORTAR` definition. Even though it is not required for the mortar contact to work, the contact null beams at the perimeter of the shell bolt hole were used to keep the simulation results similar to the explicit simulation by preserving the contact situation during hole bearing.

Merits and drawbacks

As is the case in connection type b), if the application stays within the service load regime and it is sufficient to capture the onset of a slipping motion, this modeling technique is straightforward and the connection is also easy to setup. Especially with the variant where no contact null beams are needed such that the mesh of the bolt holes can be generated using the actual hole diameter instead of accounting for the extra diameter of the contact null beams. In terms of converting the model to implicit time integration, this approach is also rather straightforward and offers possibilities to keep it compatible with or without contact null beams. Moreover, the full choice of material models that can be used to represent the behavior of the bolt may be beneficial when investigating failure in detail. However, due to the solid discretization, the critical time step in explicit simulations is typically dominated by the bolt and selective mass scaling is frequently applied. In implicit simulations, this is of course not a limiting factor. In practice, we notice that more and more users switch to this modeling technique to solve their daily problem using explicit and implicit time integration.

Initializing the Pre-Tension in the Bolt

Shanks modeled with beam elements

The preload force in a bolt discretized with a beam element in the shank is applied using

```

*INITIAL_AXIAL_FORCE_BEAM
$#      bsid      lcid      scale      kbend
        100      100
*DEFINE_CURVE
$#      lcid      sidr      sfa      sfo      offa      offo      dattyp      lcint
        100      &dtPreStr &BltForce
$#              a1              o1
              0.0              0.0
              1.0              1.0

```

Herein, `bsid` denotes the beam set ID (here 100), `lcid` is the load curve ID defining preload force versus time, `scale` is the scale factor on load curve and `kbend` is an optional bending stiffness of the bolt during the initialization phase. If the `kbend=0`, the bolt does not have a bending stiffness during initialization. In cases where this is required, the user can use this feature starting with LS-DYNA versions R10. In the presented example, a linear application of the pre-tension force `BltForce` is applied within `dtPreStr`.

The initialization phase is completed, when the load curve ends or its ordinate drops to zero. It is important to end the initialization phase before the connection is loaded in the simulation to activate the material law of the bolt. Otherwise, the normal force will still be substituted by the prescribed pre-tension. Moreover, the manual states, “the time duration of the ramp should produce a quasistatic response”. In practice of explicit simulations, like for instance during a 120 ms crash event, the bolts are very often fastened within the first millisecond, which is of course far away from producing a quasistatic response. However, settings that will help to get a quasi-static pre-tension in the bolts include sufficient damping and friction of the contact definition, application of `*DAMPING_PART_STIFFNESS` with `COEF=0.05` (more or less) and modeling the bolt connection with the smallest possible gap between the parts in contact such that unnecessary travel during the pre-tension phase is avoided.

Shanks modeled with solid elements

In comparison to beam elements, the desired direction of the pre-stress in solid elements is not so straight forward to guess. Following this, the user needs to define a cross section through the solid elements that will receive the desired initial pre-stress normal to that cross section. The easiest way of modeling such a cross section is by defining a finite circular plane via the head and the tail of the normal vector, which typically lies on the centerline of the shaft, as well as the radius of that plane, which should be a large enough to cut through all the elements in the shaft, cf. Figure 3a). Following this, the pre-tension in a bolt discretized with solid elements in the shank can be realized using

```

*INITIAL_STRESS_SECTION
$#   issid   csid   lcid   psid   vid   izshear
      100    100    100    100           1
*DEFINE_CURVE
$#   lcid   sidr   sfa   sfo   offa   offo   dattyp   lcint
      100           &dtPreStr &BltStrss
$#           a1           o1
           0.0           0.0
           1.0           1.0
*DATABASE_CROSS_SECTION_PLANE_ID
$#   csid           title
      100 Cross Section Bolt
$#   psid   xct   yct   zct   xch   ych   zch   radius
      100           -1.6           0.6           5.5
$#   xhev   yhev   zhev   lenl   lenm   id   itype

```

Herein, `issid` denotes the ID of the card, `csid` is the cross section ID at which the pre-stress is applied, `lcid` is the load curve ID defining the applied pre-stress versus time, `psid` is the part set ID of the parts that are initialized with the pre-stress and `izshear` is an optional shear stress flag, which allows shear stresses to develop inside the bolt during the initialization phase. The latter option has been revised for implicit in current

developer versions (SVN> 123041, including R11 branch) and now allows bending stresses to develop. For explicit analysis, for backward compatibility reasons, this will be available as `izshear=2` as of R11, while for implicit `izshear=1` and `izshear=2` are synonymous. In the presented example, a linear application of the pre-stress `BltStrss` is applied within `dtPreStr`.

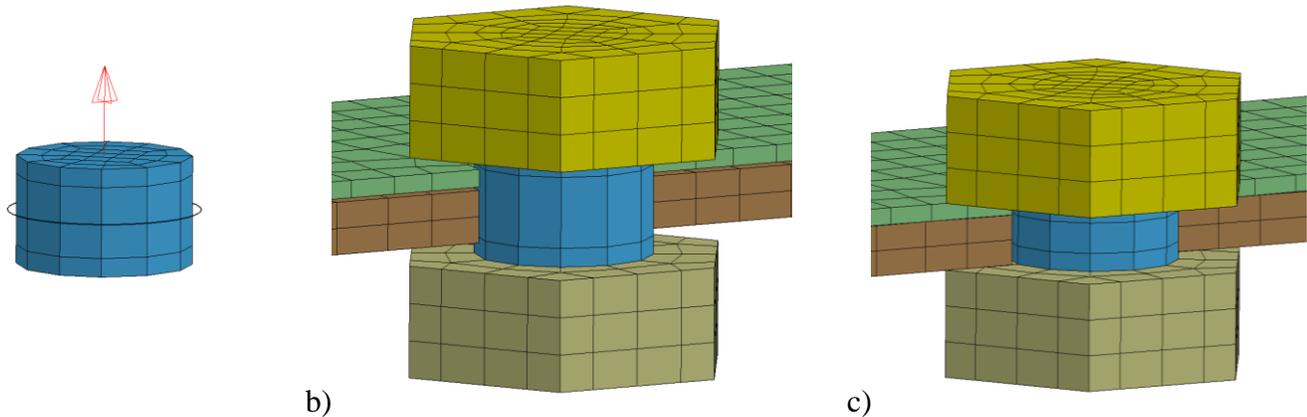


Fig. 3: a) shows the definition of the cross section plane in the shank, which is indicated by the black ring as well as the red normal vector along the centerline of the shank. Parts b) and c) show the mesh before and after the application of the pre-stress, respectively. It is apparent how the element with the pre-stress application shrink until equilibrium is reached, i.e., head and nut have traveled far enough to be in contact with the sheets.

Note how the pre-tensioning causes the bolt to shrink in Figure 3 b) and c) after the pre-stress is applied. Following this, the greater the gap between the head, nut and the sheets, the greater this initial shrinkage. If the elements where the pre-stress is applied are not big enough to compensate this movement, they will either shrink to very flat elements with a very small explicit time step or they will ultimately even shrink down to a plane, which will trigger an error termination. Similar holds when modeling the shank with a beam element.

Moreover, note how the revised optional shear stress flag `izshear` of the LS-DYNA developer version (SVN> 123041) influences the normal stress distribution in the bolt after the pre-stressing phase is over. Switching on this option allows shear stresses to develop in the solid elements during the pre-stressing phase, which leads to a more realistic distribution of the normal stresses at equilibrium, i.e., being higher at the perimeter and lower at the center of the cross section. Figure 4 shows the normal stress of the pre-stressed solid elements in the shank with (a) and without (b) the optional shear stress development.

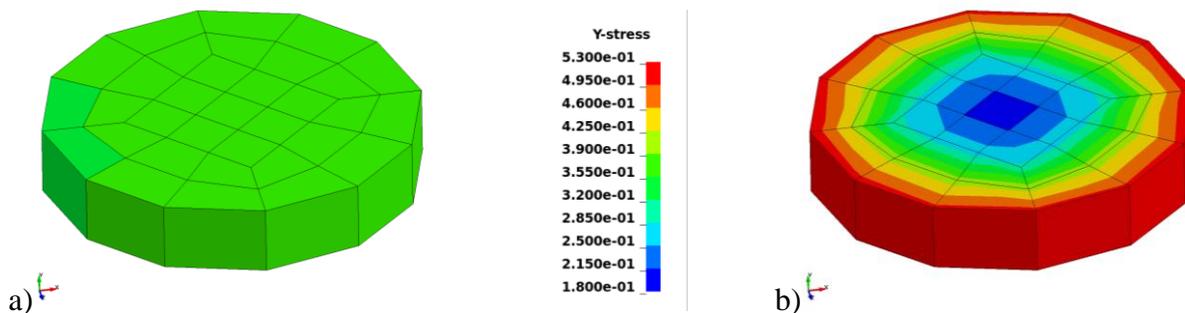


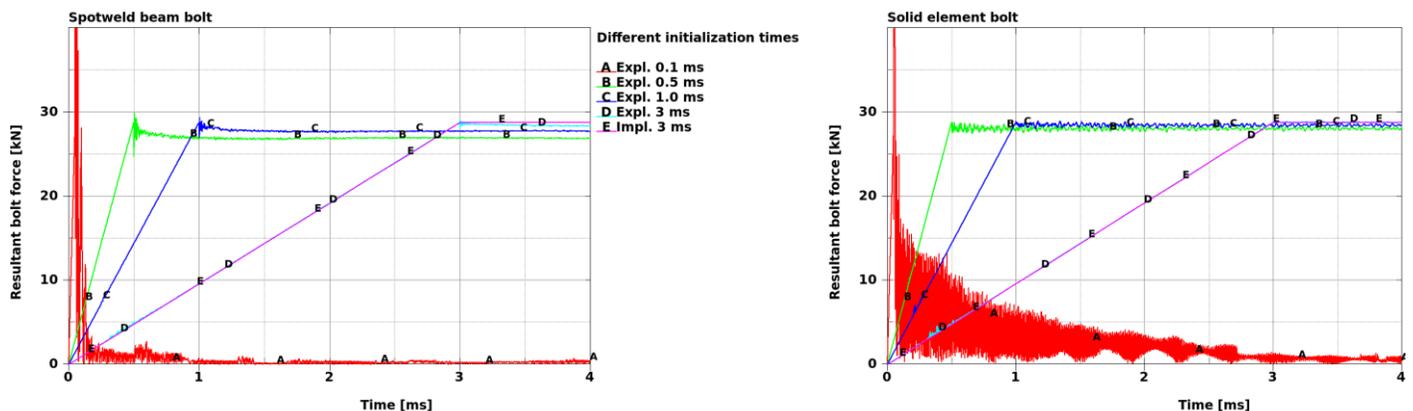
Fig. 4: Influence of the pre-stressing option `izshear` during the initialization phase of the shank with a pre-stress of 0.38 GPa in an implicit simulation using LS-DYNA developer version (SVN> 123041) where a) shows a generated homogeneous normal stress of 0.38 GPa for `izshear=0` while b) was obtained with `izshear=1` leading to an inhomogeneous normal stress which averages the desired pre-stress value of 0.38 GPa.

Finally, note that implicit simulations converge most quickly if the shank is discretized with a mesh having only hexahedron elements. In the current LS-DYNA versions R9.2 and R10.1, there seems to be a problem with pre-tensioning of pentahedrons elements whenever there is a gap between the connected sheets and the bolts head or nut. This deficiency is overcome in the LS-DYNA developer version (SVN> 123041).

General rules of thumb during pre-tension initialization

In general, the time to initialize the pre-tension in the bolt should be sufficiently long such that the head and nut of the bolt are able to establish a sufficient contact force with the connected materials, i.e., they should be in equilibrium with the pre-tension force. The amount of time needed varies with the gap that is left between the head or nut and the respective surfaces of the connected materials, i.e., the greater the gap, the further the parts have to move to get in good contact with each other and the longer it will take them to do so. If the necessary time is not given during the initialization, the bolt will not reach the desired pre-tension force when the simulation continues, as there will still be movement of the head and nut until there is equilibrium with the contact forces. Until this equilibrium is reached, the bolt will continue to contract, thereby losing its applied pre-tension again. This effect can be seen in Figure 5, where the duration of the pre-tension application of the bolt is varied.

Following this, the gap between the head, nut and the contacting materials should be as small as possible where an already closed gap resembles the perfect initial geometry. However, in practice this is not always possible to achieve such that a small gap is often inevitable but the user should model the bolt with the smallest possible gap. Another problem is caused by bolts that are modeled slightly skew due to whatever reason. In general, skew bolts should be avoided. Depending on the friction in the contact definition, skew bolts might trigger a sliding motion of the head and nut on the sheet surfaces after pre tensioning is finished, which will induce vibrations or slip of the connection.



a) Bolt modeled with spot weld beam

b) bolt modeled with solid elements

Fig. 5: Different durations for pre-tension initialization in the bolt. Curves A to D are achieved with explicit simulations while curve E is the quasi-static reference solution from an implicit simulation. It can be seen that an application of the pre-tension in only 0.1 ms is too fast and should be avoided.

Moreover, large gaps or extremely fast pre-tension applications also introduce a lot of vibration during the impact of the head and nut on the connected parts. In explicit simulations, this will cause stress waves that will continue to bounce back and forth in the rest of the simulation model causing unnecessary high noise levels. In implicit simulations, this impact might even cause the simulation not to converge properly. A close look reveals that friction in the contact definition helps to dissipate that energy and to calm the system down again. This can be greatly enhanced by applying additional damping in the initialization phase and a little bit afterwards, too. After the system calmed down, the damping can be reduced or taken away such that the load case of interest is not disturbed. Please note that global damping is not advised to use, if the model has an initial velocity (e.g. in car crash applications), as the global damping will slow down the whole vehicle while it is activated. Instead, `*DAMPING_PART_STIFFNESS` of 5% might be more suitable solution to keep oscillations to a min-

imum. Figure 5 shows how the force in the bolt oscillates after the head and nut impact the materials to be connected.

In typical models for explicit crash analysis, the bolts are modelled with the smallest possible gap and a pre-tension initialization phase of 0.5 to 1 ms. Thus, in an explicit crash simulation, the pre-tensioning of the bolts is easily accommodated in the whole simulation run.

Implicit models benefit from the possibility of applying larger time steps and the ability to reduce dynamic effects such that static equilibrium is reached, thereby fully calming down the pre-tensioned model. However, all implicit simulations where bolts are pre-tensioned should start fully dynamic to avoid a singular stiffness matrix while the bolt is still loose in the bolt hole. Damped Newmark time integration schemes as well as decaying dynamic terms help to reach a quasi-static solution after the pre-tension initialization phase. Typically, this can be achieved with the following control cards, where gamma and beta govern the numerical damping of the Numark scheme and curve ID 42 starts to switch off dynamic contributions at time $dtPreStr$ to achieve a quasi-static solution at time $2*dtPreStress$. If the simulation needs to be continued as a static simulation, this is sufficient. In the case of the bolt with loads beyond the service load, a slipping motion will occur that should be tackled by switching on the dynamic parts again. This can be done by adding a fourth coordinate in curve ID 42 which reaches the ordinate of 1.0 after the time $\&tLoad$. Please note that values between 0.0 and 1.0 are also permitted if one is interested to keep the system as calm as possible.

```

*CONTROL_IMPLICIT_DYNAMICS
$#   imass      gamma      beta      tdybir      tdydth      tdybur      irate      alpha
      -42        0.60      0.38000
*DEFINE_CURVE
$#   lcid      sidr      sfa      sfo      offa      offo      dattyp      lcint
      42
$#           a1           o1
              0.0           1.0
              &dtPreStr      1.0
              2.0*&dtPreStr  0.0
(           &tLoad           1.0) ... optional to switch dynamics back on

```

General Settings for the Implicit Models

All implicit simulations of bolt types a, b, c, and d were carried out using the same solver settings, which follow Appendix P in LS-DYNA[®] Keyword Manual [8]

```

*CONTROL_IMPLICIT_GENERAL
$#   imflag      dt0      imform      nsbs      igs      cnstn      form      zero_v
      1           &dt0
*CONTROL_IMPLICIT_SOLUTION
$#   nsolvr      ilimit      maxref      dctol      ectol      rctol      lstol      abstol
      12          6           12          1.0e-20
$#   dnorm      diverg      istif      nlprint      nlnorm      d3itctl      cpchk
      1           3           4           1
$#   arcctl      arcdir      arclen      arcmtl      arcdmp      arcpsi      arcalf      arctim
$#   lsmtl      lsdir      irad      srad      awgt      sred

```

Typically, the time step size `&dt0`, the number of iterations `ilimit` and the maximum number of stiffness matrix reformations `maxref` are problem specific and should be tweaked by the user to obtain the best convergence behavior. To achieve the same level of accuracy throughout the simulation, the displacement norm should be computed with respect to the displacement of the displacement increment of the last time step instead of the total displacement, which is chosen using `dnorm=1`. To prevent premature convergence, the absolute convergence criteria should be switched off by setting `abstol` to a very small number. Moreover, as shell and solid elements are present in the example, there are translational and rotational degrees of freedom to be solved. Following this, the nonlinear convergence norm should consider the sum of translational and rotational degrees of freedom, i.e., no separate treatment. This is achieved by setting `nlnorm=4`, which scales the rotational degrees of freedom to account for their different units based on a characteristic element size that is calculated internally. The remaining settings of `nlprint` and `d3itctl` are for debugging purposes to be able to identify convergence problems and can be left blank in smoothly running decks.

Another important setting addresses the automatic control of the time step size, which may depend on convergence, as well as the definition of so-called key points. The latter are necessary to define important points in time that are actually reached during the simulation, e.g. the end of the pre-tensioning phase `&dtPreStrss` or the beginning of the loading `&tLoad`. In explicit simulations, this is usually not an issue, as the time steps are small enough such that “overshooting” of key points is of negligible magnitude. In implicit simulations, time steps are larger and the overshooting may significantly miss the end of the pre-tensioning phase or the onset of a subsequent load. For the presented example, auto time stepping as well as the key points are defined by

```

*CONTROL_IMPLICIT_AUTO
$#   iauto   iteopt   itewin   dtmin   dtmax   dtexp   kfail   kcycle
      1      40      10      -24
*DEFINE_CURVE
$#   lcid     sidr     sfa     sfo     offa     offo     dattyp   lcint
      24
$#           a1           o1
      &dtPreStrss      &dtMax
      &tLoad          &dtMax

```

Herein, the optimal number of iterations is defined by the window `iteopt=40 +/- itewin=10`. If the needed iterations to converge lie above or below this window, the initial time step size `&dt0` is decreased or increased, respectively, until the boundaries `dtmin` or `dtmax` are reached. Following this, the maximum size `dtmax` of the time step can be defined by a curve definition, to alter the size during the simulation. In the presented example, this is done by the curve with ID=24 where the points of this curve simultaneously define the key points in time, which are needed for accurate results.

Example for all four Bolt Connections

The input decks of the presented four examples for friction bolt connections can be downloaded on our LS-DYNA examples page [9]. The example consists of a M10 bolt of grade 8.8, which connects two sheets of material where one sheet is modeled using shell elements and the other using hexahedron solid elements. The models were tested with the official LS-DYNA MPP R9.2 release and it has been noted that the current LS-DYNA R10 and R10.1 releases still have some issues with the implicit models. All four connection types are modeled such that their input decks run with the same control cards in explicit and implicit.

Following this, all examples are controlled by the keyword file `main.k` where the user can choose to include the needed control cards for an explicit or an implicit solution, i.e. `control_explicit.k` or `con-`

`trol_implicit.k`, respectively. The second important include to choose from is the definition of one of the four bolt types, which contains their node, element, material, section, part and set definitions as well as the initial stress, boundary and loading conditions. These include files are `bolted_connection_{a,b,c,d}.k`. To switch between bolt models as well as implicit and explicit simulations, the user should open `main.k` using a text editor and remove the comments of the respective lines in the top part of the file.

Summary

The paper provides four different modeling approaches for friction grip bolts using LS-DYNA with explicit and implicit time integration. Following this, the well-known modeling strategies in explicit have been explained, pitfalls have been identified and necessary changes to switch to implicit applications have been presented. In this process, we recognized some shortcomings and provided solutions to most of them. With the aid of the provided examples, users should be able to switch successfully to implicit time integration with minimal changes to their existing models. Moreover, it should also help to follow the one-model-strategy of LSTC when setting up new models.

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